June 2017

COUNTY OF KENOSHA
Division of Planning and Development

RECEIVED
AUG 7 2020
Kenosha County
Deputy County Clerk

RECEIVED
AUG 7 2020
Kenosha County
Planning and Development

CONDITIONAL USE PERMIT APPLICATION

(a) Property Owner's Name: Blume Transport Group LLC

Print Name: _____ Signature: _____

Mailing Address: 1490 240th Ave

City: Brighton State: WI Zip: 53139

Phone Number: 847-417-0654 E-mail (optional): _____

Note: Unless the property owner's signature can be obtained in the above space, a letter of agent status signed by the legal property owner must be submitted if you are a tenant, leaseholder, or authorized agent representing the legal owner, allowing you to act on their behalf.

(b) Agent's Name (if applicable):

Print Name: Kyle Cramer Signature: [signature]

Business Name: KJ Cramer Construction LLC

Mailing Address: 4754 S. Cottage Ln.

City: Pleasant Prairie State: WI Zip: 53158

Phone Number: 847-404-1875 E-mail (optional): KJCramerConstruction@gmail.com

(c) Architect's Name (if applicable):

Print Name: Nathan Remitz Signature: [signature]

Business Name: Patera

Mailing Address: 2601 S. Sunnyslope Rd.

City: New Berlin State: WI Zip: 53151

Phone Number: 262-786-6776 E-mail (optional): _____

(d) Engineer's Name (if applicable):

Print Name: DEVON S. ELLIS Signature: _____

Business Name: PATERA

Mailing Address: 2601 S. Sunnyslope Rd.

City: New Berlin State: WI Zip: 53151

Phone Number: 262-786-6776 ext. 101 E-mail (optional): Paul@PateraLLC.com

(e) Tax key number(s) of subject site:

45-4-221-181-0410

Address of the subject site:

20206 15th ST. Union Grove, WI 53182

(f) Plan of Operation (or attach separate plan of operation)

Type of structure:

Storage Garage

Proposed operation or use of the structure or site:

Indoor Parking for trucks currently sitting outside

Number of employees (by shift):

Hours of Operation: 06:00 - 19:00 as described in current Conditional Use Permit

Any outdoor entertainment? If so, please explain: N/A

Any outdoor storage? If so, please explain: NO

Zoning district of the property: R-2

(g) Attach a plat of survey prepared by a land surveyor registered by the State of Wisconsin or site plan drawn to scale and approved by the Department of Planning & Development showing all of the information required under section 12.05-1(h)3 for a zoning permit. In addition, the plat of survey or site plan layout shall show the location, elevation and use of any abutting lands and the location and foundation elevations of structures within 50 feet of the subject site; soil mapping unit lines; ordinary high water mark, historic high water marks and floodlands on or within 50 feet of the subject premises, and existing and proposed landscaping.

January 2013

COUNTY OF KENOSHA
Department of Planning and Development

SITE PLAN REVIEW CHECKLIST

Owner: Blume Transport Group LLC Date 6/9/2020

Mailing Address: 6480 240th Ave Brighton WI 53139 Phone # 847-417-0654

Phone # _____

Agent: KJ Kramer Construction Phone # 262-949-3486

Mailing Address: 4754 S. Cottage Ln Pleasant Prairie WI 53158 Phone # 847-404-1875

Architect/Engineer: Paul Ratajczyk Phone # 262-786-6776 ext 104

Mailing Address: 2601 S. Sunnyslope Rd. New Berlin, WI 53151 Phone # _____

Tax Parcel Number(s): 45-4-221-181.0410 Acreage of Project: 2.1

Existing Zoning: B2 Proposed Zoning: B2

Conditional Use Permit: _____

Description of Project: (include the following when applicable):

Description of project: Storage garage

Size of existing building(s): 1455 sq ft

Size of new building(s) and/or addition(s): 80x80x16

Number of current and projected full-time and part-time employees, number of shifts: 5 one Shift

Number of proposed units: _____ Description of units: _____

Density: _____

☐ Plat of Survey Submitted:

☐ Covenants and Restrictions Submitted

LIGHTING PLAN

We will be placing three Photo Controlled Hubbell model number NRG-356L-4K-U-PC on west face of building (1) above each overhead door. We will also be Placing one Hubbell NRG-356L-5K-U light on south side of building that will be on a switch and not photo controlled. This is for general lighting/ security lighting of the building. Light projecting roughly one parking space of depth from the building. Manufacturer fixture specifications provided.

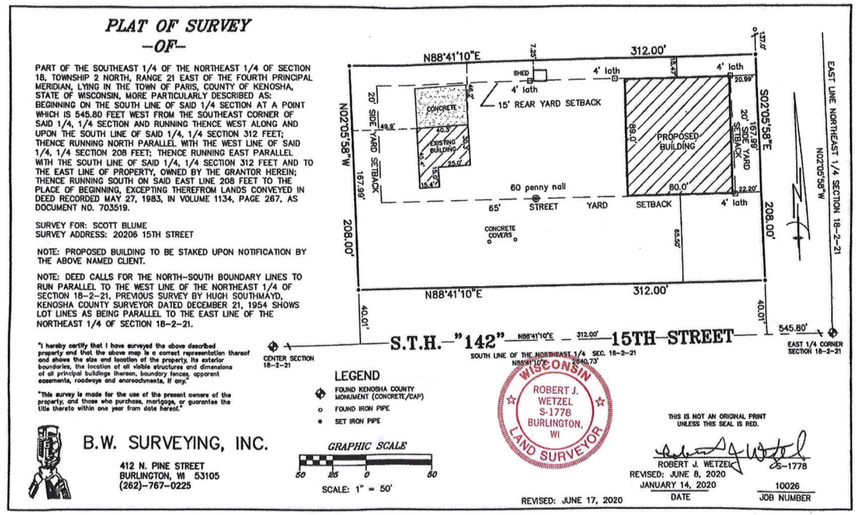
PLAT OF SURVEY
-OF-

PART OF THE SOUTHEAST 1/4 OF THE NORTHEAST 1/4 OF SECTION 18, TOWNSHIP 2 NORTH, RANGE 21 EAST OF THE FOURTH PRINCIPAL MERIDIAN, LYING IN THE TOWN OF PARIS, COUNTY OF KENOSHA, STATE OF WISCONSIN, MORE PARTICULARLY DESCRIBED AS: BEGINNING ON THE SOUTH LINE OF SAID 1/4 SECTION AT A POINT WHICH IS 545.80 FEET WEST FROM THE SOUTHEAST CORNER OF SAID 1/4, 1/4 SECTION AND RUNNING THENCE WEST ALONG AND UPON THE SOUTH LINE OF SAID 1/4, 1/4 SECTION 312 FEET; THENCE RUNNING NORTH PARALLEL WITH THE WEST LINE OF SAID 1/4, 1/4 SECTION 208 FEET; THENCE RUNNING EAST PARALLEL WITH THE SOUTH LINE OF SAID 1/4, 1/4 SECTION 312 FEET AND TO THE EAST LINE OF PROPERTY, OWNED BY THE GRANTOR HEREIN; THENCE RUNNING SOUTH ON SAID EAST LINE 208 FEET TO THE PLACE OF BEGINNING, EXCEPTING THEREFROM LANDS CONVEYED IN DEED RECORDED MAY 27, 1983, IN VOLUME 1134, PAGE 267, AS DOCUMENT NO. 703519.

SURVEY FOR: SCOTT BLUME
SURVEY ADDRESS: 20206 15TH STREET

NOTE: PROPOSED BUILDING TO BE STAKED UPON NOTIFICATION BY THE ABOVE NAMED CLIENT.

NOTE: DEED CALLS FOR THE NORTH-SOUTH BOUNDARY LINES TO RUN PARALLEL TO THE WEST LINE OF THE NORTHEAST 1/4 OF SECTION 18-2-21. PREVIOUS SURVEY BY HUGH SOUTHWAYD, KENOSHA COUNTY SURVEYOR DATED DECEMBER 21, 1954 SHOWS LOT LINES AS BEING PARALLEL TO THE EAST LINE OF THE NORTHEAST 1/4 OF SECTION 18-2-21.



LEGEND
- FOUND KENOSHA COUNTY MONUMENT (CONCRETE/CAP)
- FOUND IRON PIPE
- SET IRON PIPE

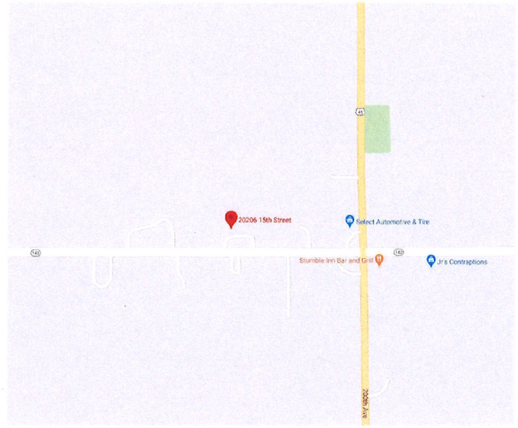
GRAPHIC SCALE
50 25 0 50
SCALE: 1" = 50'

WISCONSIN
ROBERT J. WETZEL
S-1778
BURLINGTON, WI
LAND SURVEYOR

THIS IS NOT AN ORIGINAL PRINT UNLESS THIS SEAL IS RED.

ROBERT J. WETZEL S-1778
REVISED: JUNE 8, 2020
JANUARY 14, 2020 (DATE)
10026 (JOB NUMBER)
REVISED: JUNE 17, 2020

B.W. SURVEYING, INC.
412 N. PINE STREET
BURLINGTON, WI 53105
(262)-767-0225



1 PROJECT LOCATION
S.00 NTS

GENERAL NOTES:

1. THE DESIGNER MAINTAINS NO RESPONSIBILITY FOR THE GENERAL CONTRACTOR, SUBCONTRACTORS, OR THOSE WORKING IN SUCH CAPACITIES, FOR THE METHODS USED, OR LACK THEREOF, IN THE EXECUTION OF THE WORK AND SAFETY PROCEDURES AND PRECAUTIONS TAKEN AT THE PROJECT SITE.
2. CONTRACTORS SHALL ASSUME FULL RESPONSIBILITY - UNRELIEVED BY REVIEW OF SHOP DRAWINGS NOR BY SUPERVISION OR PERIODIC OBSERVATION OF CONSTRUCTION FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS - FOR DIMENSIONS TO BE CONFIRMED AND CORRELATED ON THE JOB SITE AND BETWEEN INDIVIDUAL DRAWINGS OR SETS OF DRAWINGS; FOR FABRICATION PROCESSES AND CONSTRUCTION TECHNIQUES (INCLUDING EXCAVATION, SHORING AND SCAFFOLDING, BRACING, ERECTION, FORM WORK, ETC.) FOR COORDINATION OF THE VARIOUS TRADES; FOR SAFE CONDITIONS ON THE JOB SITE; AND FOR THE PROTECTION OF THE PEOPLE AND PROPERTY AT THE JOB SITE.
3. THE INFORMATION CONTAINED ON THE DRAWINGS IS IN ITSELF INCOMPLETE, AND VOID UNLESS USED IN CONJUNCTION WITH ALL THE SPECIFICATIONS, TRADE PRACTICES, OR APPLICABLE STANDARDS, CODES, ETC., INCORPORATED THEREIN BY REFERENCE, OF WHICH THE CONTRACTOR CERTIFIES KNOWLEDGE BY SIGNING THE CONTRACT.
4. UNLESS NOTED OTHERWISE, ALL DETAILS, SECTIONS, AND NOTES ON THE DRAWINGS ARE INTENDED TO BE TYPICAL FOR SIMILAR SITUATIONS ELSEWHERE
5. UNLESS OTHERWISE SHOWN OR NOTED, THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE LOCATION AND THE PLACEMENT OF ANY INSERTS, HANGARS, PIPE SLEEVES, HOLES OR ANCHOR BOLTS THAT ARE REQUIRED BY THE MECHANICAL OR ELECTRICAL EQUIPMENT.
6. THE CONTRACTOR SHALL COMPLY WITH THE LATEST OCCUPATIONAL SAFETY HEALTH ACT REQUIREMENTS.
7. ALL STATE OF WISCONSIN, LOCAL AND O.S.H.A. SAFETY CODES SHALL BE A PART OF THESE PLANS, AND IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO SEE THAT ALL PARTIES THAT WORK AT OR VISIT THE JOB SITE COMPLY WITH SAME.

BUILDING DATA :

2015 IBC

USE AND OCCUPANCY CLASSIFICATION: (Chapter 3)	"S-1" LOW-HAZARD STORAGE (NON-OCCUPIED STORAGE)	
ALLOWABLE AREA AND HEIGHT:	"S-1 (V-B)" TWO STORIES - 9,000 sq. ft.	
ACTUAL AREA:	TOTAL BUILDING AREA:	7,120 sq. ft.
SPRINKLERS:	NONE	
CONSTRUCTION TYPE: (Table-601)	"V-B"	
FIRE RATINGS (per table 601 & 602)		
STRUCTURAL FRAME:	0 - HR. RATING	
BEARING WALLS EXTERIOR:	0 - HR. RATING	
BEARING WALLS INTERIOR:	0 - HR. RATING	
NON-BEARING WALLS EXTERIOR:	0 - HR. RATING	
NON- BEARING WALLS INTERIOR:	0 - HR. RATING	
FLOOR CONSTRUCTION:	0 - HR. RATING	
ROOF CONSTRUCTION:	0 - HR. RATING	
EXIT TRAVEL DISTANCE: (Table 1017.2)	300 feet	
COMMON PATH OF TRAVEL: (per TABLE 1006.2.1)	75 feet	
TOTAL OCCUPANCY LOADING (per TABLE 1004.1.1)	TOTAL BUILDING OCCUPANTS: (1 PER 200 S.F.)	36 OCCUPANTS

ABBREVIATIONS:

A.B.	ANCHOR BOLT	EA	EACH	L	STEEL ANGLE	SIM.	SIMILAR
&	AND	E.J.	EXPANSION JOINT	lbs.	POUNDS	SPEC.	SPECIFICATION
@	AT	ELEC.	ELECTRICAL	L.L.	LIVE LOAD	S.O.G.	SLAB ON GRADE
ADJ.	ADJACENT	ELEV.	ELEVATION	L.L.H.	LONG-LEG HORIZONTAL	SQ.	SQUARE
ADTL.	ADDITIONAL	ENG.	ENGINEER	L.L.V.	LONG-LEG VERTICAL	S.S.	STAINLESS STEEL
A.F.F.	ABOVE FINISHED FLOOR	EQ.	EQUAL	L.V.L.	LAMINATED VENEER LUMBER	STD.	STANDARD
ALT.	ALTERNATE	EQUIP.	EQUIPMENT	LHT.	LIGHT WEIGHT	STL.	STEEL
ALUM.	ALUMINUM	E.H.	EACH WAY				
APPROX.	APPROXIMATE	EXSTG	EXISTING	MAT.	MATERIAL	T.C.	TOP CHORD
ARCH.	ARCHITECT/ ARCHECTURAL	EXP.	EXPANSION	MAX.	MAXIMUM	TEMP.	TEMPERATURE
		EXT.	EXTERIOR	MECH.	MECHANICAL	T.O.	TOP OF
B.C.	BOTTOM CHORD			MIN.	MINIMUM	TYP.	TYPICAL
BLDG.	BUILDING	FOUND.	FOUNDATION	MISC.	MISCELLANEOUS		
BM	BEAM	FLR.	FLOOR	M.O.	MASONRY OPENING	U.O.N.	UNLESS OTHERWISE NOTED
BRG	BEARING	F.V.	FIELD VERIFY				
B.O.	BOTTOM OF			N.A.	NOT APPLICABLE	T.C.	TOP CHORD
		GA.	GAUGE	N.T.S	NOT TO SCALE	TEMP.	TEMPERATURE
CANT.	CANTILEVERED	GALV.	GALVANIZED	NHMT	NORMAL HEIGHT	T.O.	TOP OF
C.I.P.	CAST-IN-PLACE	G.C.	GENERAL CONTRACTO			TYP.	TYPICAL
C.J.	CONTROL JOINT			O.C.	ON CENTER		
C.L. (℄)	CENTER LINE	GRD.	GRADE	O.D.	OUTSIDE DIAMETER	W	WITH
CLG.	CEILING	GEN.	GENERIC	O.H.	OVERHEAD	W/O	WITHOUT
C.M.U.	CONCRETE MASONRY UNIT					WD	WOOD
COL.	COLUMN	HORIZ.	HORIZONTAL	pcf	POUNDS PER CUBIC FOOT	W.W.F.	WELDED WIRE FABRIC
CONC.	CONCRETE	H.S.S.	HOLLOW STRUCTURAL SECTION	pci	POUNDS PER CUBIC INCH		
CONN.	CONNECTION	HGT.	HEIGHT	plf	POUNDS PER LINEAL FOOT	'	FEET
CONT.	CONTINUOUS	HVAC	HEATING, VENTILATION, & AIR CONDITIONING	psi	POUNDS PER SQUARE INCH	"	INCH
				P.T.	PRESSURE TREATED		
DIA. (ø)	DIAMETER	I.D.	INSIDE DIAMETER	R	RADIUS		
DEMO.	DEMOLITION	INT.	INTERIOR	REIN.	REINFORCED/ REINFORCEMENT		
DIM.	DIMENSION	JST	JOIST	REQ'D	REQUIRED		
DN.	DOWN	K	KIPS	REV.	REVISION		
D.L.	DEAD LOAD	ksi	KIPS PER SQUARE INCH	R.T.U.	ROOF TOP UNIT		
DWG	DRAWING						
DWL	DOWEL						

DETAIL REFERENCE
SECTION REFERENCE
ELEVATION REFERENCE
DATUM ELEVATION (T.O. FT. 000'-0 0/0")
COLUMN GRID REFERENCE
REVISION REFERENCE
SPOT DATUM REFERENCE (ELEV. 000'-0 0/0")
BEAM DATUM ELEVATION (T.O. FTG 000'-0 0/0")
COLUMN REFERENCE (C - X)
FOOTING REFERENCE (F - X)

SCOPE OF DRAWING:

THESE DRAWINGS INDICATE THE GENERAL SCOPE OF THE PROJECT IN TERMS OF ARCHITECTURAL DESIGN INTENT, THE DIMENSIONS OF THE BUILDING, THE MAJOR ARCHITECTURAL ELEMENTS AND THE TYPE OF STRUCTURAL, MECHANICAL AND ELECTRICAL SYSTEMS. THE DRAWINGS DO NOT NECESSARILY INDICATE OR DESCRIBE ALL WORK REQUIRED FOR FULL PERFORMANCE AND COMPLETION OF THE REQUIREMENTS OF THE CONTRACT. ON THE BASIS OF GENERAL SCOPE INDICATED OR DESCRIBED, THE TRADE CONTRACTORS SHALL FURNISH ALL ITEMS REQUIRED FOR THE PROPER EXECUTION AND COMPLETION OF THE WORK INTENDED.

ARCHITECTURAL:	STRUCTURAL:

SHEET INDEX:

PATERA LLC
2601 S. Sunny Slope Rd. • New Berlin, WI 53151
Office: 262.786.6776 Fax: 262.786.7036

REVISIONS:
- PROGRESS SET: 7.16.20
- ARCH. REVIEW SET: 7.29.20
-
-
-

BLUME POLE BUILDING

20206 15TH ST
UNION GROVE, WI 53182

SHEET TITLE:
PROPOSED SITE PLAN, BUILDING DATA & INDEX SHEET

S.001

DATE: JULY 29TH, 2020

PROJECT NUMBER: 20-263

DESIGN CODE:

WISCONSIN COMMERCIAL BUILDING CODE & THE ADOPTED INTERNATIONAL BUILDING CODE

BUILDING DESIGN LIVE LOADS/CRITERIA:

OCCUPANT LIVE LOADS:
- SLAB ON GRADE ..... 250 PSF

ROOF LOAD DESIGN INFORMATION:
- GROUND SNOW LOAD (pg) ..... 30 PSF
- TERRAIN CATEGORY C
- EXPOSURE CONDITION ..... FULLY EXPOSED
- EXPOSURE FACTOR (Ce) 0.90
- THERMAL FACTOR (Ct) 1.20
- IMPORTANCE FACTOR (Is) 0.80
- FLAT ROOF SNOW LOAD (pf) ..... 16.63 PSF
  * SEE DRIFTED SNOW LOAD DIAGRAMS FOR ADDITIONAL LOADING INFORMATION *

WIND LOAD DESIGN INFORMATION:
- BASIC WIND SPEED ..... 115 mph
- RISK CATEGORY I
- VELOCITY PRESSURE COEFFICIENT (Kd) 0.85
- EXPOSURE CATEGORY C
- ENCLOSURE CLASSIFICATION ..... PARTIALLY ENCLOSED
- INTERNAL PRESSURE COEFFICIENT (GCpi) ..... ±0.55
- GUST EFFECT FACTOR 0.85
- TOPOGRAPHIC FACTOR (Kzt) 1.0

SEISMIC DESIGN INFORMATION:
- RISK CATEGORY I
- SITE CLASS (ASSUMED) ..... D
- Ss 0.102
- S1 ..... 0.051
- Sms 0.162
- Sm1 ..... 0.123
- Sds 0.108
- Sd1 ..... 0.082
- SEISMIC DESIGN CATEGORY ..... B

MATERIAL STRENGTHS:

CONCRETE (COMPRESSIVE STRENGTH @ 28 DAYS):
- SLAB ON GRADE ..... f'c = 4,000 PSI
- FOOTINGS & ISOLATED PADS ..... f'c = 3,000 PSI
- FOUNDATION WALLS ..... f'c = 3,000 PSI

REINFORCING STEEL:
- BILLET A615 GRADE 60 ..... Fy = 60,000 PSI

STRUCTURAL STEEL:
- ROLLED "WIDE FLANGE" SHAPES A992
- TUBE SHAPES "HOLLOW STRUCTURAL SECTION" ..... A500 GR. B
- ALL OTHER ROLLED SHAPES & PLATES A36

STRUCTURAL BOLTS:
- ANCHOR BOLTS (HOT-DIP GALV.) A325
- THROUGH BOLT CONNECTIONS A325
- THREADED RODS A36

WELDED CONNECTIONS:
- WELDING ELECTRODES ..... E70xx (70 KSI)

MASONRY MORTAR:
- TYPE "M" MORTAR BELOW GRADE
- TYPE "M" OR "S" ABOVE GRADE

WOOD FRAMING:
- 2X8, 2X10 & 2X12 MEMBERS TO BE DOUGLAS-FIR LARCH #2 (NORTH)
  APPLIES TO PRESSURE TREATED MEMBERS AS WELL:
  - Fb (BENDING) ..... 850 PSI
  - Fv (SHEAR) ..... 180 PSI
  - Fcp (COMP. PERP. TO GRAIN) ..... 625 PSI
  - Fc (COMP. PARA. TO GRAIN) ..... 1,400 PSI
  - Ft (TENSION PARA. TO GRAIN) ..... 500 PSI
  - E (MODULUS OF ELASTICITY) ..... 1,600,000 PSI

- 2X4 MEMBERS TO BE SPRUCE PINE-FIR #2:
  - Fb (BENDING) ..... 875 PSI
  - Fv (SHEAR) ..... 135 PSI
  - Fcp (COMP. PERP. TO GRAIN) ..... 425 PSI
  - Fc (COMP. PARA. TO GRAIN) ..... 1,150 PSI
  - Ft (TENSION PARA. TO GRAIN) ..... 425 PSI
  - E (MODULUS OF ELASTICITY) ..... 1,400,000 PSI

- 2X6 MEMBERS TO BE MACHINE STRESS RATED:
  - Fb (BENDING) ..... 2,400 PSI
  - Fv (SHEAR) ..... 180 PSI
  - Fcp (COMP. PERP. TO GRAIN) ..... 625 PSI
  - Fc (COMP. PARA. TO GRAIN) ..... 1,475 PSI
  - Ft (TENSION PARA. TO GRAIN) ..... 1,925 PSI
  - E (MODULUS OF ELASTICITY) ..... 2,000,000 PSI

- LAMINATED-VENEER LUMBER (LVL), ROSEBURG MANUFACTURE:
  - Fb (BENDING) ..... 3,100 PSI
  - Fv (SHEAR) ..... 290 PSI
  - Fcp (COMP. PERP. TO GRAIN, EDGEWISE) ..... 750 PSI
  - Fc (COMP. PARA. TO GRAIN) ..... 3,000 PSI
  - Ft (TENSION PARA. TO GRAIN) ..... 2,100 PSI
  - E (MODULUS OF ELASTICITY) ..... 2,000,000 PSI

SOIL BEARING (ASSUMED TO BE FIELD VERIFIED):
- ALLOWABLE BEARING PRESSURE ..... 2,000 PSF

GENERAL:

1. ALL MATERIALS, CONSTRUCTION, AND DETAILS SHALL CONFORM WITH THE FOLLOWING: PLANS AND SPECIFICATIONS, BUILDING CODE INDICATED ABOVE & OSHA REGULATIONS.
2. THE GENERAL CONTRACTOR AND SUBCONTRACTORS SHALL BE FAMILIAR WITH THE ENTIRE SET OF CONSTRUCTION DOCUMENTS (ARCHITECTURAL, CIVIL, ELECTRICAL, PLUMBING, STRUCTURAL, ETC.) IN ORDER TO PROVIDE ALL CONSTRUCTION AND MATERIALS FOR THIS PROJECT.
3. THE CONTRACTOR SHALL REFER TO OTHER DRAWINGS CONTAINED IN THE CONSTRUCTION DOCUMENTS FOR ADDITIONAL SPECIFIED MEMBERS, DIMENSIONS, ELEVATIONS, DETAILS, OPENINGS, INSERTS, SLEEVES, DEPRESSIONS, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS REQUIRED TO CONSTRUCT THIS PROJECT.
4. DETAILS SHOWN ON STRUCTURAL DRAWINGS SHALL BE APPLICABLE TO ALL PORTIONS OF THE CONTRACT DOCUMENTS UNLESS NOTED OTHERWISE.
5. DIMENSIONS AND ELEVATIONS SHOWN ON ARCHITECTURAL DRAWINGS SUPERSEDE DIMENSIONS AND ELEVATIONS SHOWN ON STRUCTURAL DRAWINGS.
6. DO NOT SCALE PLANS.
7. IN NO CASE SHALL STRUCTURAL ALTERATIONS OR WORK AFFECTING A STRUCTURAL MEMBER BE MADE UNLESS APPROVED BY THE STRUCTURAL DESIGNER OF RECORD.
8. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND CONSTRUCTION SEQUENCE IN ORDER TO INSURE THE SAFETY OF THE BUILDING AND WORKMEN DURING CONSTRUCTION (MEANS & METHODS OF CONSTRUCTION). THIS INCLUDES, BUT IS NOT LIMITED TO: SHORING, UNDERPINNING, TEMPORARY BRACING, ETC.
9. CONSTRUCTION DOCUMENTS SHOW DIMENSIONS AND ELEVATIONS TO SIGNIFICANT WORKING POINTS (COLUMN CENTERLINES, OUTSIDE FACE OF WALLS, TOP OF FRAMING MEMBERS, ETC.) MATERIAL SUPPLIERS AND DESIGNERS ARE RESPONSIBLE FOR ALL OTHER INFORMATION IN ORDER TO DETAIL/FABRICATE THEIR WORK. CONTACT THE ARCHITECT WITH ANY DISCREPANCIES.
10. IN THE EVENT OF ANY DISCREPANCIES BETWEEN THE STRUCTURAL DRAWINGS AND ANY OTHER PLANS CONTAINED IN THIS SET OF CONSTRUCTION DOCUMENTS, THE CONTRACTOR SHALL BRING THE DISCREPANCY TO THE ARCHITECTS ATTENTION IN WRITING IMMEDIATELY OR SHALL BID THE MOST EXPENSIVE INSTALLATION SPECIFIED.

FOUNDATION & EARTHWORK:

1. ALL EXTERIOR FOOTINGS MUST BEAR AT A MINIMUM DEPTH OF 4'-0" BELOW ADJACENT FINISH EXTERIOR GRADE.
2. DO NOT PLACE ANY FOOTINGS ON FROZEN SUBGRADE.
3. BACK FILLING SHALL BE DONE SIMULTANEOUSLY ON BOTH SIDES OF FOUNDATION WALLS.
4. DO NOT PLACE BACK FILL AGAINST BASEMENT WALLS UNTIL THE TOP AND BOTTOM OF THE WALL ARE ADEQUATELY BRACED BY THE SLAB ON GRADE AND THE FLOOR FRAMING AT THE TOP OF THE WALL.
5. REMOVE ANY EXISTING CONCRETE 2'-0" BELOW NEW CONCRETE FOOTINGS AND SLABS ON GRADE.
6. SHORING/OR UNDERPINNING SHALL BE DESIGNED TO LIMIT HORIZONTAL AND VERTICAL MOVEMENT OF EXISTING CONSTRUCTION TO 1/4" MAXIMUM IN ANY DIRECTION.
7. CENTER PIER AND COLUMN FOOTINGS ON COLUMN CENTERLINES AND WALL FOOTINGS ON WALL CENTERLINES UNLESS SPECIFICALLY NOTED OTHERWISE.
8. ALL BACK FILL WITHIN 3'-0" OF RETAINING WALLS AND BASEMENT WALLS SHALL BE FREE DRAINING GRANULAR MATERIAL APPROVED BY A SOILS ENGINEER AND COMPACTED TO 90% STANDARD PROCTOR.
9. TOP OF FOOTING ELEVATIONS SHOWN ON THESE CONSTRUCTION DOCUMENTS REPRESENT MINIMUM FOOTING DEPTHS FOR FROST PROTECTION AND BEST JUDGMENT OF A SUITABLE BEARING STRATUM. ACTUAL GRADE CONDITIONS AND SUITABLE BEARING STRATUM MUST BE VERIFIED BY THE CONTRACTOR AND A SOILS ENGINEER AT THE TIME OF EXCAVATION.
10. FOOTING EXCAVATIONS MUST EXTEND TO COMPETENT BEARING MATERIAL. CONTRACTOR SHALL HIRE A SOILS ENGINEER TO FIELD VERIFY NET ALLOWABLE SOIL BEARING CAPACITY STATED ON THESE CONSTRUCTION DOCUMENTS AND IN GEOTECHNICHAL REPORT FOR THIS PROJECT. IF SUITABLE BEARING STRATUM DOES NOT EXIST AT FOOTING ELEVATIONS STATED ON CONSTRUCTION DOCUMENTS, EXCAVATIONS SHALL BE EXTENDED UNTIL SOIL WITH STATED BEARING CAPACITY IS REACHED. PLACE COMPACTED FILL BELOW FOOTINGS OR EXTEND FOOTINGS DOWN TO SUITABLE BEARING STRATUM. ENGINEERED FILL BELOW SLABS ON GRADE AND FOOTINGS SHALL BE FREE DRAINING GRANULAR MATERIAL COMPACTED TO 95% MODIFIED PROCTOR AND PLACED PER THE SOIL ENGINEERS RECOMMENDATIONS.
11. REFER TO SOILS REPORT (IF APPLICABLE) FOR DESCRIPTION OF EXISTING SOIL CONDITIONS AND RECOMMENDATIONS.
12. WHERE NEW FOOTINGS ABUT EXISTING FOOTINGS, STEP THE NEW FOOTING AS REQUIRED TO HAVE NEW BOTT/FTG ELEVATION MATCH EXISTING BOTT/FTG ELEVATION. CONTRACTOR SHALL FIELD VERIFY EXISTING BOTT/FTG ELEVATION.

CAST-IN-PLACE REINFORCED CONCRETE:

1. CONCRETE WORK SHALL CONFORM TO ACI 318 (BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE).
2. CONTRACTOR SHALL SUBMIT A SET OF STEEL REBAR SHOP DRAWINGS FOR APPROVAL PRIOR TO CONSTRUCTION. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING TO THE ARCHITECT.
3. MAXIMUM WATER/CEMENT RATIO FOR CONCRETE SHALL BE AS FOLLOWS:
   - 0.41 FOR SLABS ON GRADE
   - 0.54 FOR CONCRETE BELOW GRADE
   - 0.42 FOR EXPOSED CONCRETE
4. CONCRETE EXPOSED TO EXTERIOR CONDITIONS SHALL BE AIR-ENTRAINED 4%-6%.
5. GROUT BELOW BASE PLATES AND BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT 3/4" THICK MINIMUM.
6. STEEL REINFORCING BARS SHALL CONFORM TO ASTM A615 (GRADE 60). DEFORMED WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
7. CONTRACTOR SHALL PROVIDE SUITABLE WIRE SPACERS, CHAIRS, TIES, ETC FOR SUPPORTING REINFORCING STEEL IN THE PROPER POSITION WHILE PLACING CONCRETE.
8. PROVIDE (2)-#5 BARS AROUND ALL OPENINGS AND (2)-#5 BARS DIAGONALLY AT ALL OPENING CORNERS. EXTEND BARS 2'-6" PAST OPENING.
9. PROVIDE 1/2" EXPANSION JOINT MATERIAL AT INTERIOR LOCATIONS WHERE SLABS ABUT WALLS, COLUMNS, AND OTHER VERTICAL SURFACES UNLESS NOTED OTHERWISE.
10. PROVIDE A 1" CHAMFER ON EXPOSED CORNERS OF CONCRETE UNLESS NOTED OTHERWISE.
11. DO NOT PLACE CONDUITS, PIPES, DUCTS, OR FIXTURES IN STRUCTURAL CONCRETE UNLESS NOTED OTHERWISE.
12. SLEEVES, CONDUITS, OR PIPING PASSING THROUGH CONCRETE SLABS AND WALLS SHALL BE PLACED SO THAT THEY ARE NOT CLOSER THAN THREE DIAMETERS ON CENTER AND SO THAT THEY DO NOT DISPLACE REINFORCING.
13. PROVIDE SAW CUT CONTROL JOINTS IN CONCRETE SLABS ON METAL DECK SPACED NO MORE THAN 20'-0" APART. PLACE CONTROL JOINTS ON COLUMN CENTER LINES IN EACH DIRECTION. REFER TO CONTROL JOINT LAYOUT SHOWN ON FOUNDATION PLAN FOR REFERENCE.
14. CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIR OF ANY IRREGULARITIES OR DEFECTS IN CONCRETE SLABS (CRACKS, BUMPS, FLOOR CURLING, ETC.) BEFORE ANY FLOOR FINISHES ARE APPLIED.
15. REFER TO REINFORCEMENT DEVELOPMENT AND LAP SPLICE SCHEDULE FOR LAP SPLICES IN REINFORCING STEEL.
16. ALL LAPS IN REINFORCING STEEL SHALL BE CLASS "B" LAP SPLICES UNLESS OTHERWISE NOTED.
17. CONCRETE TEST REPORTS SHALL DIRECTLY STATE WHETHER OR NOT THE TEST RESULT COMPLIES WITH THE CONSTRUCTION DOCUMENTS AND SPECIFICATIONS.
18. MAXIMUM SLUMP FOR ALL CONCRETE SHALL NOT EXCEED 4".
19. CLASS C FLY ASH OR SLAG MAY BE SUBSTITUTED FOR CEMENT ON A POUND TO POUND BASIS UP TO 10% OF THE TOTAL CEMENTITIOUS CONTENT.
20. ALL CONCRETE SLABS SHALL BE WET CURED PER ACI RECOMMENDATIONS FOR NO LESS THAN SEVEN DAYS.
21. CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE ARE NOT PERMITTED IN ANY CONCRETE MIX.
22. PROVIDE THE FOLLOWING CLEAR COVER DISTANCES FOR REINFORCEMENT IN CONCRETE UNLESS OTHERWISE NOTED:

    FOOTINGS - ALL SIDES ..... 3"
    SLABS NOT PERMANENTLY AGAINST EARTH - BOTTOM & SIDES ..... 1"
    SLABS PERMANENTLY AGAINST EARTH - BOTTOM & SIDES ..... 3"
    SLABS - TOP ..... 3/4"
    WALLS NOT PERMANENTLY AGAINST EARTH ..... 1"
    WALLS PERMANENTLY AGAINST EARTH ..... 3"
    BEAMS & GIRDERS NOT PERMANENTLY AGAINST EARTH ..... 1 1/2"
    BEAMS & GIRDERS NOT PERMANENTLY AGAINST EARTH ..... 1 1/2"
    PIERS & COLUMNS NOT PERMANENTLY AGAINST EARTH ..... 2"

23. CONTRACTOR SHALL USE SMOOTH FORMS FOR EXPOSED CONCRETE SURFACES. ANY CONCRETE SURFACE REPAIRS SHALL BE PERFORMED BY THE CONTRACTOR AS REQUIRED. REPAIR AND PATCH DEFECTIVE AREAS WITH PROPRIETARY PATCHING COMPOUND IMMEDIATELY AFTER REMOVAL OF FORMS.

STRUCTURAL STEEL:

1. DESIGN, FABRICATION, AND ERECTION SHALL CONFORM TO THE CURRENT EDITION OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) "MANUAL OF STEEL CONSTRUCTION".
2. STEEL DETAILING AND CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT EDITION OF AISC "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN".
3. WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS HOLDING CURRENT AWS CERTIFICATES IN THE TYPES OF WELDING SPECIFIED ON THESE CONSTRUCTION DOCUMENTS.
4. CONTRACTOR SHALL SUBMIT FIVE SETS OF STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL DESIGNER OF RECORD.
5. CONTRACTOR SHALL DESIGN AND PROVIDE ANY TEMPORARY BRACING OR GUYS REQUIRED TO ERECT STEEL MEMBERS. TEMPORARY BRACING SHALL BE LEFT IN PLACE UNTIL THE PERMANENT STRUCTURE IS IN PLACE AND SECURE.
6. PROVIDE 3/16" CAP PLATE AT THE ENDS OF ALL EXPOSED TUBE AND PIPE MEMBERS.
7. STAIRS, HANDRAILS, AND GUARDRAILS SHALL BE DESIGNED BY THE STEEL SUPPLIER OR ARCHITECT OR RECORD, VERIFY ROLES AND RESPONSIBILITY.
8. ALL STEEL BEAMS SHALL BE FABRICATED WITH THE NATURAL CAMBER (WITHIN MILL TOLERANCE).
9. THE STEEL SUPPLIER SHALL COORDINATE HIS WORK WITH THE STEEL JOIST SUPPLIER ON THE PROJECT (IF APPLICABLE, SEE CONSTRUCTION DOCUMENTS).
10. CAPACITY OF BOLTED OR WELDED CONNECTIONS SHALL BE EQUAL TO OR EXCEED 120% OF BEAM REACTION PRODUCED BY MAXIMUM ALLOWABLE UNIFORM LOAD ON THE GIVEN MEMBER SPAN.
11. UNLESS OTHERWISE NOTED ON CONSTRUCTION DOCUMENTS, ALL BEAM CONNECTIONS SHALL BE DOUBLE ANGLE CONNECTIONS WITH A325 BOLTS. AT BEAM TO BEAM AND BEAM TO COLUMN CONNECTIONS, PROVIDE AS MANY BOLTS AS POSSIBLE IN BEAM FLANGE. DOUBLE ANGLE WELDED CONNECTIONS MAY BE USED TO DEVELOP THE SAME CAPACITY AS A BOLTED CONNECTION.
12. ALTERNATE CONNECTIONS FROM WHAT IS SPECIFIED ON THE CONSTRUCTION DOCUMENTS WILL NOT BE ACCEPTED WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL DESINGER OF RECORD.
13. USE STANDARD AISC DOUBLE ANGLE CONNECTIONS WHERE POSSIBLE. ALL STANDARD DOUBLE ANGLE CONNECTIONS SHALL BE IN ACCORDANCE WITH ASD CURRENT EDITION AND SHALL BE TYPE 2 FRAMING CONNECTIONS UNLESS NOTED OTHERWISE.
14. WHERE WOOD MEMBERS FRAME INTO STEEL MEMBERS, PROVIDE A SADDLE CONNECTION. SEE CONSTRUCTION DOCUMENTS AND/ OR VERIFY CONNECTION WITH STRUCTURAL DESIGNER OF RECORD.
15. PROVIDE STIFFENER PLATES ON BOTH SIDES OF BEAM WEBS AT ALL CONCENTRATED LOADS ABOVE AND BELOW A BEAM. UNLESS NOTED OTHERWISE, FRAME THE LARGEST BEAM OVER COLUMNS AT BEAM TO BEAM INTERSECTIONS.

WOOD FRAMING:

1. DESIGN, FABRICATION, AND CONSTRUCTION SHALL CONFORM TO THE CURRENT EDITION OF "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", AMERICAN FOREST AND PAPER ASSOCIATION.
2. DESIGN, FABRICATION, AND CONSTRUCTION OF ALL PLYWOOD/ O.S.B. FRAMING SHALL CONFORM TO THE CURRENT EDITION OF "PLYWOOD DESIGN SPECIFICATIONS", AMERICAN PLYWOOD ASSOCIATION.
3. PLYWOOD/ O.S.B. SHEATHING SHALL CONFORM TO THE CURRENT EDITION OF "U.S. PRODUCT STANDARD PS-1" FOR SOFTWOOD PLYWOOD/ O.S.B. AND BEAR THE APA GRADE-TRADEMARK OF THE AMERICAN PLYWOOD ASSOCIATION.
4. PLYWOOD/ O.S.B. SHEATHING SHALL BE ATTACHED TO WOOD FRAMING WITH "STRENGTH AXIS PARALLEL TO FLOOR. STAGGER ALL JOINTS.
5. PLYWOOD/ O.S.B. SHEATHING SHALL BE FASTENED TO SUPPORTS W/ 8d NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS UNLESS NOTED OTHERWISE.
6. ANY PLYWOOD/ O.S.B. SHEATHING THAT IS EXPOSED TO MOISTURE SHALL BE PRESSURE TREATED.
7. PLYWOOD/ O.S.B. PANEL EDGES SHALL BEAR ON THE FRAMING SUPPORT MEMBERS AND BUTT ALONG THEIR CENTER LINES. NAILS SHALL BE PLACED NOT LESS THAN 3/8" IN FROM THE PANEL EDGE.
8. WOOD SILL PLATES AND OTHER WOOD MEMBERS DIRECTLY EXPOSED TO MOISTURE OR IN DIRECT CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED.
9. MAXIMUM MOISTURE CONTENT IN ANY WOOD MEMBER SHALL NOT EXCEED 19%.
10. 2x WOOD JOISTS SHALL HAVE 1x3 SPF NO.2 CROSS BRIDGING AT 8'-0" o/c MAXIMUM.
11. DO NOT EMBED NON-PRESSURE TREATED WOOD MEMBERS IN CONCRETE.
12. ALL BOLTS AND LAG SCREWS SHALL CONFORM TO ASTM. USE STEEL WASHERS BETWEEN HEAD OF BOLT OR LAG SCREW AND WOOD. USE STEEL WASHERS BETWEEN NUT AND WOOD. VERIFY GALVANIC REACTION CONDITIONS AND REQUIREMENTS
13. ALL FASTENERS ATTACHING PRESSURE TREATED WOOD MEMBERS TO CONCRETE OR MASONRY SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL.
14. MAKE NO SUBSTITUTIONS OF ANY ENGINEERED WOOD PRODUCTS (LVL, PSL, LSL, ETC.) SPECIFIED ON ANY FRAMING PLANS WITH OUT THE DIRECT WRITTEN PERMISSION OF THE STRUCTURAL DESIGNER OF RECORD.

METAL PLATE CONNECTED WOOD TRUSS NOTES/CRITERIA:

1. WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE CURRENT EDITIONS OF "DESIGN SPECIFICATIONS FOR METAL PLATE CONNECTED WOOD TRUSSES" BY TRUSS PLATE INSTITUTE (TPI) AND "NATIONAL DESIGN SPECIFICATIONS FOR STRESS-GRADE LUMBER AND ITS FASTENINGS" BY NATIONAL FOREST PRODUCTS ASSOCIATION.
2. ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS:
   - TOP CHORD LIVE LOAD ..... * SEE DRIFTED SNOW LOADING *
   - TOP CHORD DEAD LOAD ..... 10 PSF
   - BOTTOM CHORD LIVE LOAD ..... N/A (VERIFY NON-STORAGE LOAD)
   - BOTTOM CHORD DEAD LOAD ..... 10 PSF
3. FLOOR TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS:
   - TOP CHORD LIVE LOAD ..... N/A
   - TOP CHORD DEAD LOAD ..... N/A
   - BOTTOM CHORD LIVE LOAD ..... N/A
   - BOTTOM CHORD DEAD LOAD ..... N/A
4. IN ADDITION TO THE LOADS STATED ABOVE THE TRUSSES SHALL BE DESIGNED FOR ANY SNOW DRIFTING, MECHANICAL, AND/OR ANY SPECIAL LOAD CONDITIONS AS SHOWN ON STRUCTURAL OR ARCHITECTURAL PLANS.
5. ROOF TRUSSES SHALL BE DESIGNED FOR A MAXIMUM LIVE LOAD DEFLECTION OF L/240.
6. FLOOR TRUSSES SHALL BE DESIGNED FOR A MAXIMUM LIVE LOAD DEFLECTION OF L/480.
7. FABRICATION, HANDLING, STORAGE, AND ERECTION SHALL BE IN ACCORDANCE WITH "TRUSS PLATE INSTITUTION" RECOMMENDED PRACTICES AND SHALL BE DONE IN A WORKMAN LIKE MANNER SO AS TO NOT DAMAGE THE TRUSSES. TRUSSES SHALL NOT BE CUT, ADDED INTO OR ALTERED IN ANY WAY WITH OUT THE WRITTEN CONSENT OF THE TRUSS DESIGNER, STRUCTURAL DESIGNER, OR ARCHITECT OF RECORD.
8. WOOD TRUSS DESIGNER/SUPPLIER SHALL SUBMIT FORMAL STAMPED CALCULATIONS BY A REGISTERED ENGINEER FOR REVIEW BEFORE FABRICATION.
9. SUBMIT A SET OF TRUSS SHOP DRAWINGS TO THE ARCHITECT & STRUCTURAL DESIGNER OF RECORD FOR APPROVAL PRIOR TO FABRICATION. CONTRACTOR SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING TO THE ARCHITECT.
10. SHOP DRAWING SUBMISSIONS SHALL INCLUDE THE FOLLOWING INFORMATION:
    - THE NAME, ADDRESS, PHONE NUMBER, AND FAX NUMBER OF THE SUPPLIER.
    - SLOPE OR DEPTH, SPAN AND SPACING
    - LOCATION OF ALL JOINTS
    - ALL DESIGN LOADS
    - ADJUSTMENTS TO LUMBER AND METAL CONNECTOR PLATE VALUES FOR CONDITIONS OF USE
    - EACH REACTION FORCE AND DIRECTION
    - METAL CONNECTOR PLATE TYPE, SIZE, GAUGE, AND THE DIMENSIONAL LOCATION OF EACH CONNECTOR PLATE
    - LUMBER SIZE, SPECIES, AND GRADE FOR EACH TRUSS MEMBER
    - CONNECTION REQUIREMENTS FOR TRUSS TO TRUSS GIRDER, TRUSS PLY TO PLY, AND FIELD SPLICES
    - CALCULATED DEFLECTION RATIO AND/OR MAXIMUM DEFLECTION FOR LIVE AND TOTAL LOAD
    - SPECIFY ALL TRUSS TO TRUSS CONNECTIONS AND HANGERS.
    - SPECIFY AND SHOW ALL PERMANENT TRUSS BRACING REQUIRED BY DESIGN.
11. CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION PROCEDURES AND TEMPORARY TRUSS BRACING REQUIREMENTS DURING ERECTION IN ACCORDANCE WITH TPI's COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING, AND BRACING METAL PLATE CONNECTED WOOD TRUSSES (HIB-91 BOOKLET) AND THE CURRENT EDITION OF ANSI/TPI-1.
12. TRUSSES EXPOSED TO MOISTURE SHALL BE CONSTRUCTED OF PRESSURE TREATED WOOD AND GALVANIZED METAL PLATES.
13. FLOOR TRUSS SPACING SHOWN ON FRAMING PLANS ARE MAXIMUM SPACINGS. TRUSS DESIGNER SHALL REDUCE SPACING AS REQUIRED TO SUPPORT ALL LOADS SPECIFIED ON THESE PLANS AND BY CODE.
14. DESIGN ROOF TRUSSES TO RESIST ALL WIND LOADS INCLUDING UPLIFT LOADS (SEE WIND LOADING DIAGRAMS FOR INFORMATION).
15. ALL TRUSS TO TRUSS CONNECTIONS ARE TO BE DESIGNED, DETAILED, AND SUPPLIED BY THE TRUSS SUPPLIER/ DESIGNER.
16. TRUSS FABRICATOR SHALL FIELD VERIFY ALL SPAN DIMENSIONS BEFORE FABRICATING.

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REVISIONS:
- PROGRESS SET: 7.16.20
- ARCH. REVIEW SET: 7.29.20
-
-
-

BLUME POLE BUILDING

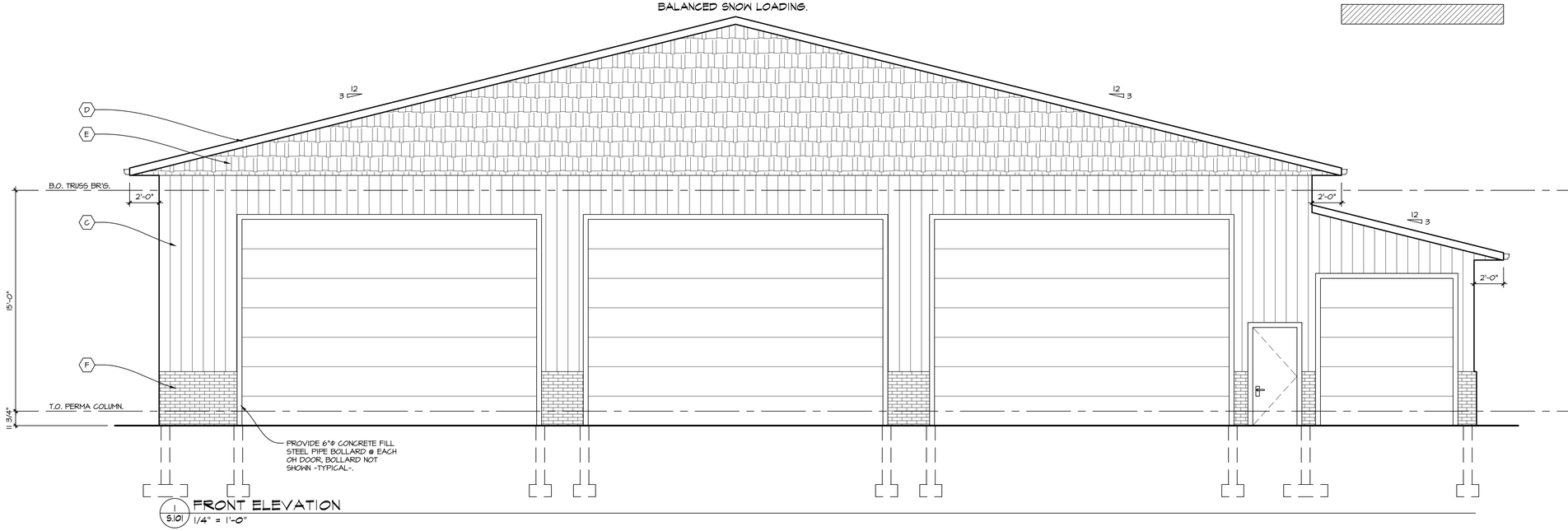
20206 15TH ST
UNION GROVE, WI 53182

SHEET TITLE: STRUCTURAL SPECIFICATIONS

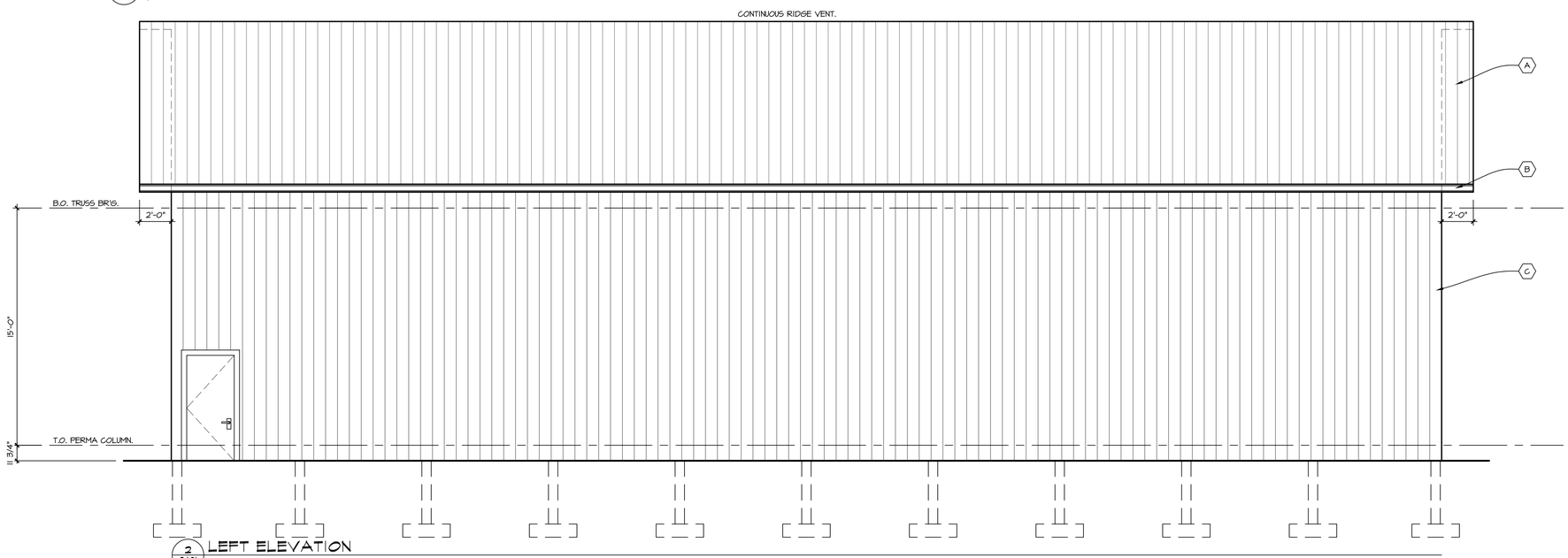
S.002

DATE: JULY 29TH, 2020

PROJECT NUMBER: 20-263



1 S.101 FRONT ELEVATION
1/4" = 1'-0"



2 S.101 LEFT ELEVATION
1/4" = 1'-0"

ELEVATION TAGS:

- A — 29 GAUGE PRE-PAINTED METAL ROOF PANELS.
- B — 6" STEEL CLAD FASCIA W/ ALUMINUM GUTTERS & DOWNSPOUTS.
- C — 29 GAUGE PRE-PAINTED METAL SIDING (VERTICAL).
- D — 6" STEEL CLAD FASCIA.
- E — VINYL SHAKE.
- F — MASONRY WAINSCOTING.
- G — 29 GAUGE PRE-PAINTED METAL SIDING (VERTICAL) WAINSCOTING.

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BLUME POLE BUILDING

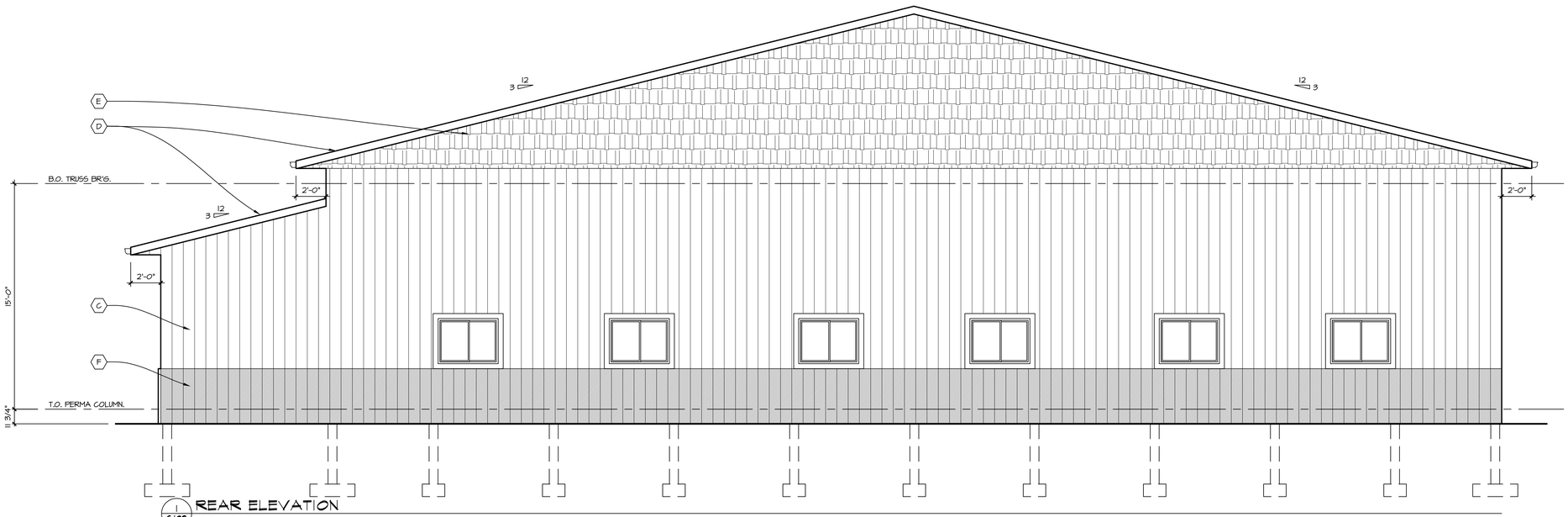
20206 15TH ST
UNION GROVE, WI 53182

SHEET TITLE: ELEVATIONS

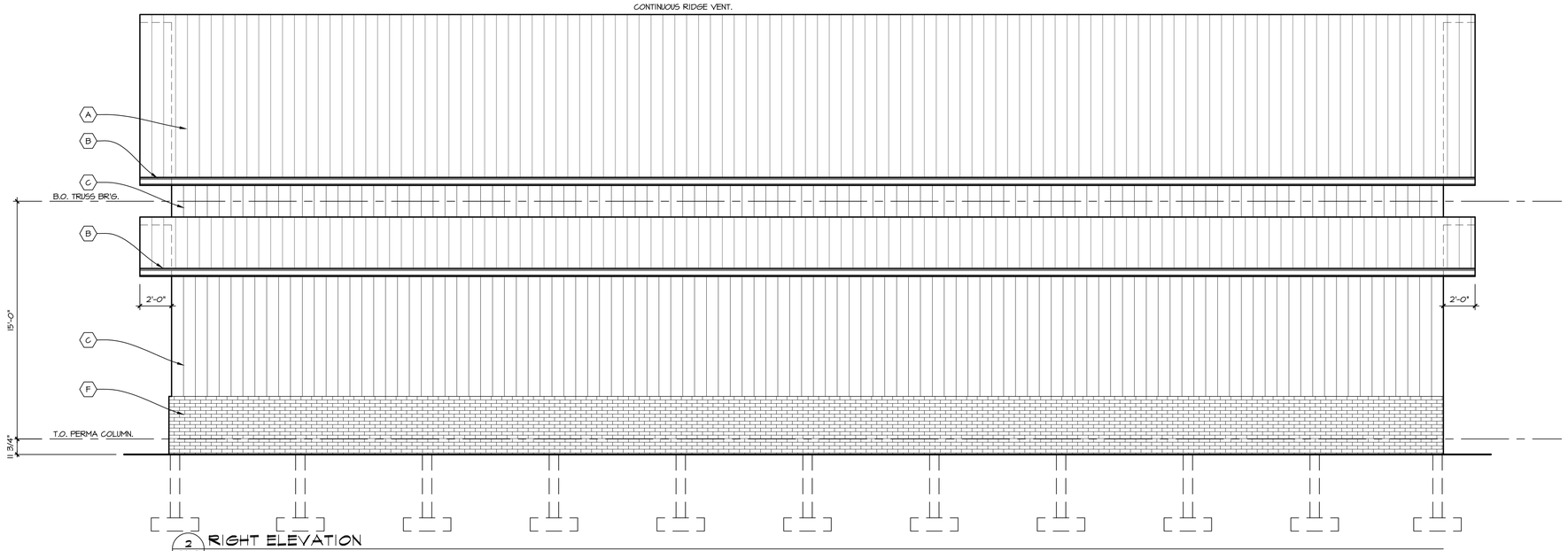
S.101

DATE: JULY 29TH, 2020

PROJECT NUMBER: 20-263



1 / S.102 REAR ELEVATION
1/4" = 1'-0"



2 / S.102 RIGHT ELEVATION
1/4" = 1'-0"

ELEVATION TAGS:

- A — 29 GAUGE PRE-PAINTED METAL ROOF PANELS.
- B — 6" STEEL CLAD FASCIA W/ ALUMINUM GUTTERS & DOWNSPOUTS.
- C — 29 GAUGE PRE-PAINTED METAL SIDING (VERTICAL).
- D — 6" STEEL CLAD FASCIA.
- E — VINYL SHAKE.
- F — MASONRY WAINSCOTING.
- G — 29 GAUGE PRE-PAINTED METAL SIDING (VERTICAL) WAINSCOTING.

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BLUME POLE BUILDING

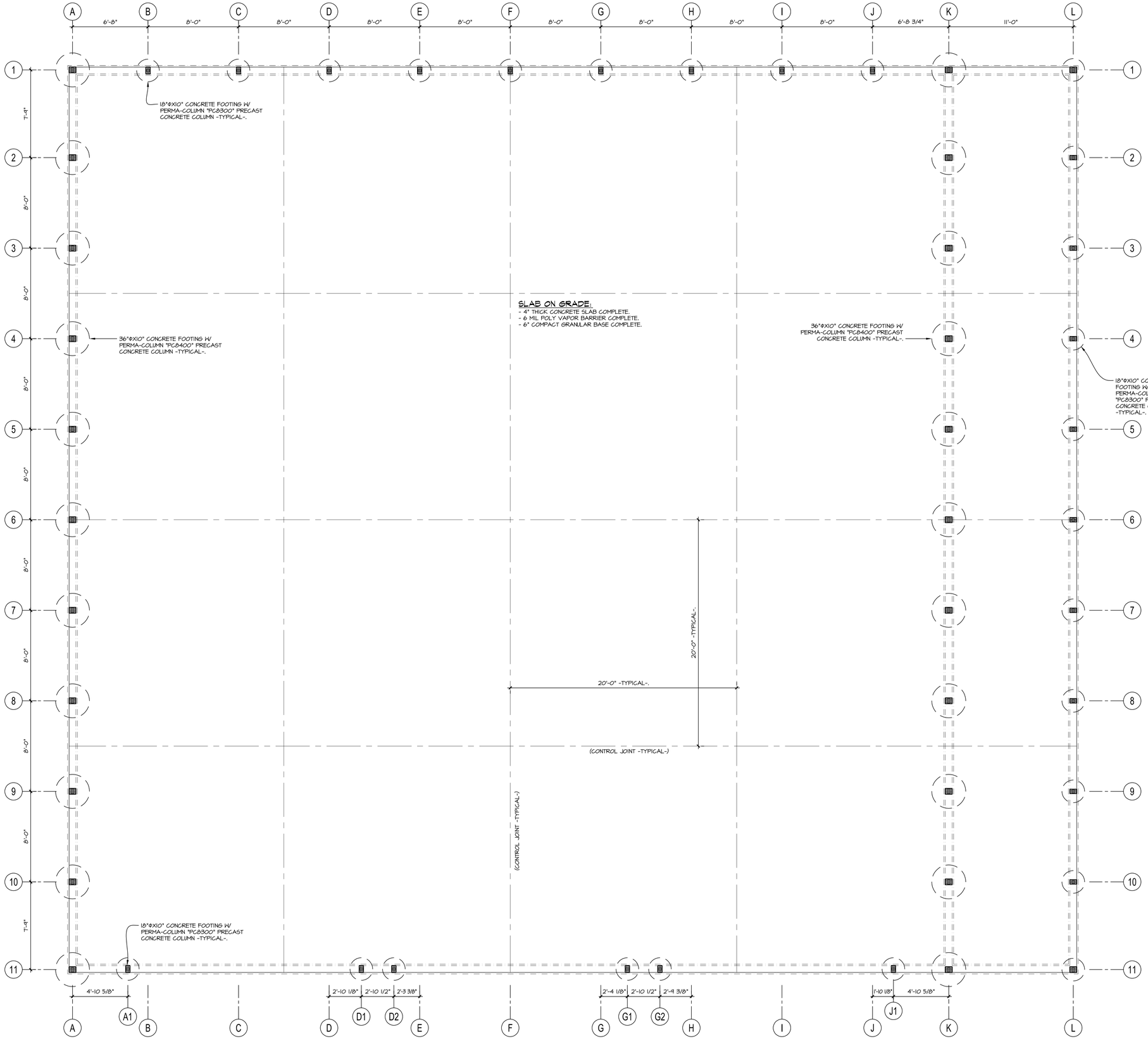
20206 15TH ST
UNION GROVE, WI 53182

SHEET TITLE: ELEVATIONS

S.102

DATE: JULY 29TH, 2020

PROJECT NUMBER: 20-263



1 S.201 FOUNDATION PLAN
1/4" = 1'-0"

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-

BLUME POLE BUILDING

20206 15TH ST
UNION GROVE, WI 53182

SHEET TITLE:
FOUNDATION PLAN

S.201

DATE: JULY 29TH, 2020

PROJECT NUMBER: 20-263

ROOF DECKING:
- PROVIDE 24 GAUGE ROOF PANELS COMPLETE & ATTACH W/ #10 SCREWS @ 36/4 PATTERN.

(2) 2X8 ABOVE & BELOW WINDOW -TYPICAL-.

3 / S.301

A 6'-8" B 8'-0" C 8'-0" D 8'-0" E 8'-0" F 8'-0" G 8'-0" H 8'-0" I 8'-0" J 6'-8 3/4" K 11'-0" L

1 7'-4" 1a 3'-10 3/4" 2 8'-0" 3 8'-0" 4 8'-0" 5 8'-0" 6 8'-0" 7 8'-0" 8 8'-0" 9 8'-0" 10 7'-4" 11

3'-0"X6'-8" DOOR.
(EXIT)
FE
PROVIDE ADA THRESHOLD & LEVER HARDWARE.

4'-0"X3'-0" SLIDER WINDOW.

GIRDER TRUSS.

2X6 (MSR) PURLINS @ 24" O.C. -TYPICAL-

SIMPSON "LU26" HANGER @ 16" O.C. -TYPICAL-.

SIMPSON "LU26" HANGER @ 24" O.C. -TYPICAL-.

2X6 (MSR) PURLINS @ 16" O.C. @ LOWER ROOF -TYPICAL-

(COMMON PATH OF TRAVEL = 44'-10")

4 / S.301

(4) 2X8 GIRDER TRUSS COLUMN -TYPICAL-.

(ROOF RIDGE LINE ABOVE)

(3) 2X8 GIRDER TRUSS COLUMN -TYPICAL-.

5 / S.301

CONTINUOUS LEDGER BOARD TO BEAR LOWER ROOF GIRDER TRUSSES, SEE SECTION FOR SPECS.

(COMMON PATH OF TRAVEL = 48'-1")

(1) 2X8 SHOULDER W/ (2) 2X8 KING STUDS.

20'X14' OVERHEAD DOOR.

9'X10' OVERHEAD DOOR.

4'-10 5/8" | 1'-9 3/8" | 2'-10 1/8" | 2'-10 1/2" | 2'-3 3/8" | 2'-4 1/8" | 2'-10 1/2" | 2'-9 3/8" | 1'-10 1/8"

A A1 B C D D1 D2 E 3 / S.301 F G G1 G2 H I J J1 K L

(2) 11 7/8" LVL HEADER.

(2) 2X8 HEADER.

(2) 2X12 HEADER.

ROOF FRAMING PLAN

1 / S.202 1/4" = 1'-0"

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-

BLUME POLE BUILDING

20206 15TH ST
UNION GROVE, WI 53182

SHEET TITLE:
ROOF FRAMING PLAN

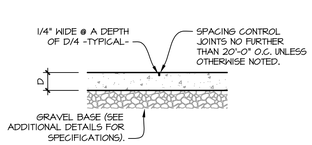
S.202

DATE: JULY 29TH, 2020

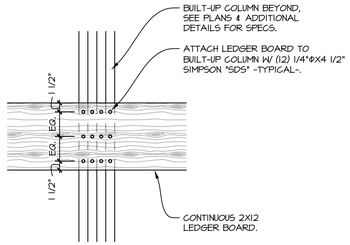
PROJECT NUMBER: 20-263

IBC TABLE 2304.9.1 FASTENING SCHEDULE FOR WOOD CONSTRUCTION:

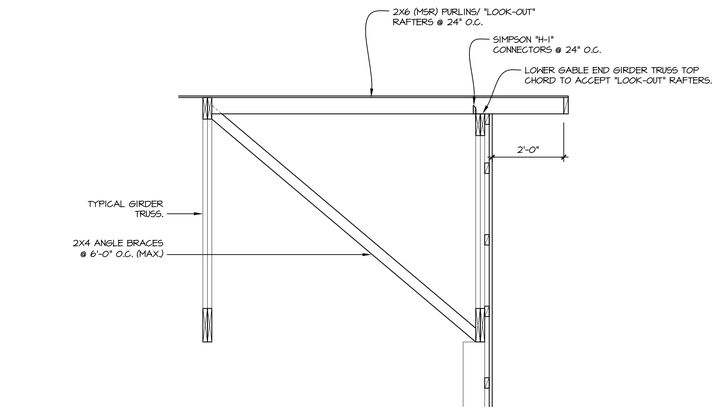
CONNECTION	FASTENING	LOCATION
JOIST TO SILL OR GIRDER	(3) 8d COMMON (2 1/2"X0.131") (3) 3"X0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAIL
BRIDGING TO JOIST	(2) 8d COMMON (2 1/2"X0.131") (2) 3"X0.131" NAILS (2) 3" 14 GAGE STAPLES	TOENAIL EACH END
1"X6" SUB-FLOOR OR LESS TO EACH JOIST	(2) 8d COMMON (2 1/2"X0.131")	FACE NAIL
WIDER THAN 1"X6" SUB-FLOOR TO EACH JOIST	(3) 8d COMMON (2 1/2"X0.131")	FACE NAIL
2" SUB-FLOOR TO JOIST OR GIRDER	(2) 16d COMMON (3 1/2"X0.162")	BLIND & FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING	16d COMMON (3 1/2"X0.162") @ 16" O.C. 3"X0.131" NAILS @ 8" O.C. 3" 14 GAGE STAPLE @ 12" O.C.	TYPICAL FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING @ BRACED WALL PANEL	(3) 16d COMMON (3 1/2"X0.162") @ 16" O.C. (4) 3"X0.131" NAILS @ 16" O.C. (4) 3" 14 GAGE STAPLE @ 16" O.C.	TYPICAL FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING @ BRACED WALL PANEL	(3) 16d COMMON (3 1/2"X0.162") @ 16" O.C. (4) 3"X0.131" NAILS @ 16" O.C. (4) 3" 14 GAGE STAPLE @ 16" O.C.	BRACED WALL PANELS
TOP PLATE TO STUD	(2) 16d COMMON (3 1/2"X0.162") (3) 3"X0.131" NAILS (3) 3" 14 GAGE STAPLE	END NAIL
DOUBLE STUDS	16d COMMON (3 1/2"X0.162") @ 24" O.C. 3"X0.131" NAILS @ 12" O.C. 3" 14 GAGE STAPLE @ 8" O.C.	FACE NAIL
DOUBLE TOP PLATES	16d COMMON (3 1/2"X0.162") @ 16" O.C. 3"X0.131" NAILS @ 12" O.C. 3" 14 GAGE STAPLE @ 12" O.C.	TYPICAL FACE NAIL
DOUBLE TOP PLATES (SPLICE/ LAP)	(8) 16d COMMON (3 1/2"X0.162") (12) 3"X0.131" NAILS (12) 3" 14 GAGE STAPLE	LAP SPLICE
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	(3) 8d COMMON (2 1/2"X0.131") (3) 3"X0.131" NAILS (3) 3" 14 GAGE STAPLE	TOENAIL
RIM JOIST TO TOP PLATE	8d COMMON (2 1/2"X0.131") @ 6" O.C. 3"X0.131" NAILS @ 6" O.C. 3" 14 GAGE STAPLE @ 6" O.C.	TOENAIL
TOP PLATES, LAP & INTERSECTIONS	(2) 16d COMMON (3 1/2"X0.162") (3) 3"X0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAIL
CONTINUOUS HEADER, TWO PIECES	16d COMMON (3 1/2"X0.162")	16" O.C. ALONG EDGE
CEILING JOISTS TO PLATE	(3) 8d COMMON (2 1/2"X0.131") (5) 3"X0.131" NAILS (5) 3" 14 GAGE STAPLES	TOENAIL
CEILING JOISTS, LAP OVER PARTITIONS	(3) 16d COMMON (3 1/2"X0.162") (4) 3"X0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL
CEILING JOISTS TO PARALLEL RAFTERS	(3) 16d COMMON (3 1/2"X0.162") (4) 3"X0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL
RAFTER TO PLATE	(3) 8d COMMON (2 1/2"X0.131") (3) 3"X0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAIL
1" DIAGONAL BRACE TO EACH STUD END & PLATE	(2) 8d COMMON (2 1/2"X0.131") (2) 3"X0.131" NAILS (3) 3" 14 GAGE STAPLES	FACE NAIL
1"X8" SHEATHING TO EACH BEARING	(3) 8d COMMON (2 1/2"X0.131")	FACE NAIL
WIDER THAN 1"X8" SHEATHING TO EACH BEARING	(3) 8d COMMON (2 1/2"X0.131")	FACE NAIL
BUILT-UP CORNER STUDS	16d COMMON (3 1/2"X0.162") @ 24" O.C. 3"X0.131" NAILS @ 16" O.C. 3" 14 GAGE STAPLES @ 16" O.C.	FACE NAIL
BUILT-UP GIRDER & BEAMS	20d COMMON (4"X0.192") @ 32" O.C. 3"X0.131" NAILS @ 24" O.C. 3" 14 GAGE STAPLES @ 24" O.C. (2) 20d COMMON (4"X0.192") (3) 3"X0.131" NAILS (3) 3" 14 GAGE STAPLES	FACE NAIL @ TOP & BOTTOM STAGGERED ON OPPOSITE SIDES FACE NAIL @ ENDS & @ EACH SPLICE
2" PLANKS	16d COMMON (3 1/2"X0.162")	@ EACH BEARING
COLLAR TIE TO RAFTER	(3) 10d COMMON (3"X0.148") (4) 3"X0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL
JACK RAFTER TO HIP	(3) 10d COMMON (3"X0.148") (4) 3"X0.131" NAILS (4) 3" 14 GAGE STAPLES (2) 16d COMMON (3 1/2"X0.162") (4) 3"X0.131" NAILS (4) 3" 14 GAGE STAPLES	TOENAIL FACE NAIL
ROOF RAFTER TO 2-BY RIDGE BEAM	(2) 16d COMMON (3 1/2"X0.162") (3) 3"X0.131" NAILS (3) 3" 14 GAGE STAPLES (2) 16d COMMON (3 1/2"X0.162") (3) 3"X0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAIL FACE NAIL
JOIST TO BAND JOIST	(3) 16d COMMON (3 1/2"X0.162") (4) 3"X0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL
LEDGER STRIP	(3) 16d COMMON (3 1/2"X0.162") (4) 3"X0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL @ EACH JOIST



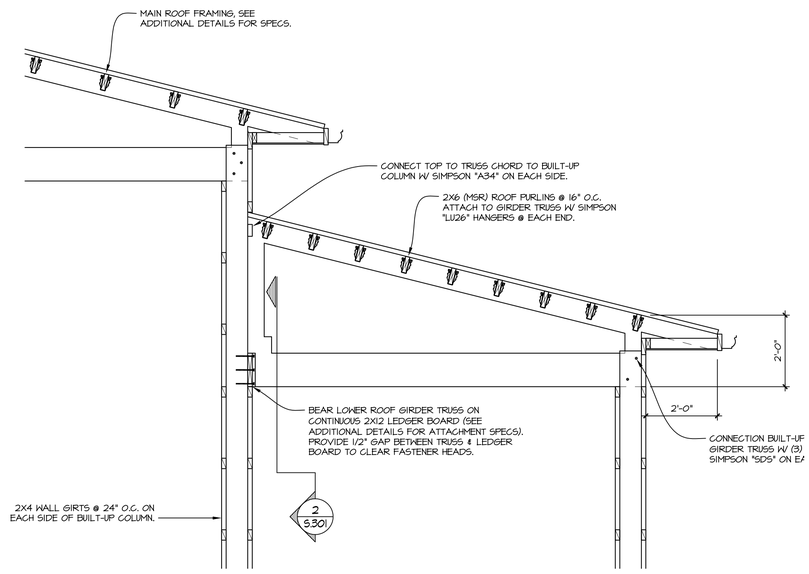
1 S.301 SLAB CONTROL JOINT
1/2" = 1'-0"



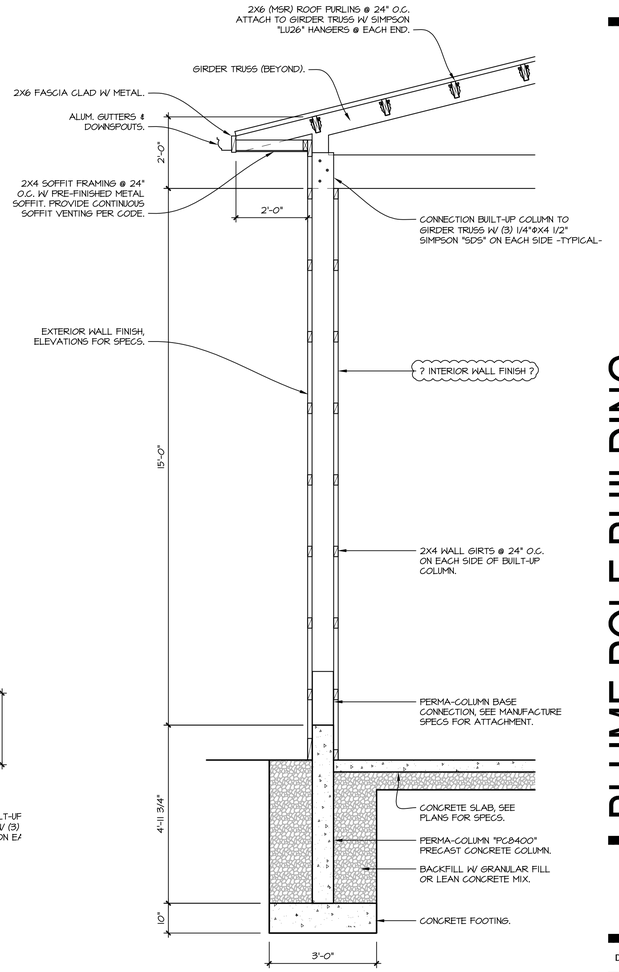
2 S.301 LEDGER BOARD ATTACHMENT
1" = 1'-0"



3 S.301 GABLE TRUSS SECTION
1/2" = 1'-0"



4 S.301 LOWER ROOF SECTION
1/2" = 1'-0"



5 S.301 WALL SECTION
1/2" = 1'-0"

PATERA LLC
2601 S. Sunny Slope Rd • New Berlin, WI 53151
Office: 262.786.6776 fax: 262.786.7036

REVISIONS:
- PROGRESS SET: 7.16.20
- ARCH. REVIEW SET: 7.29.20
-
-

BLUME POLE BUILDING

20206 15TH ST
UNION GROVE, WI 53182

SHEET TITLE: SECTIONS & DETAILS

S.301

DATE: JULY 29TH, 2020

PROJECT NUMBER: 20-263

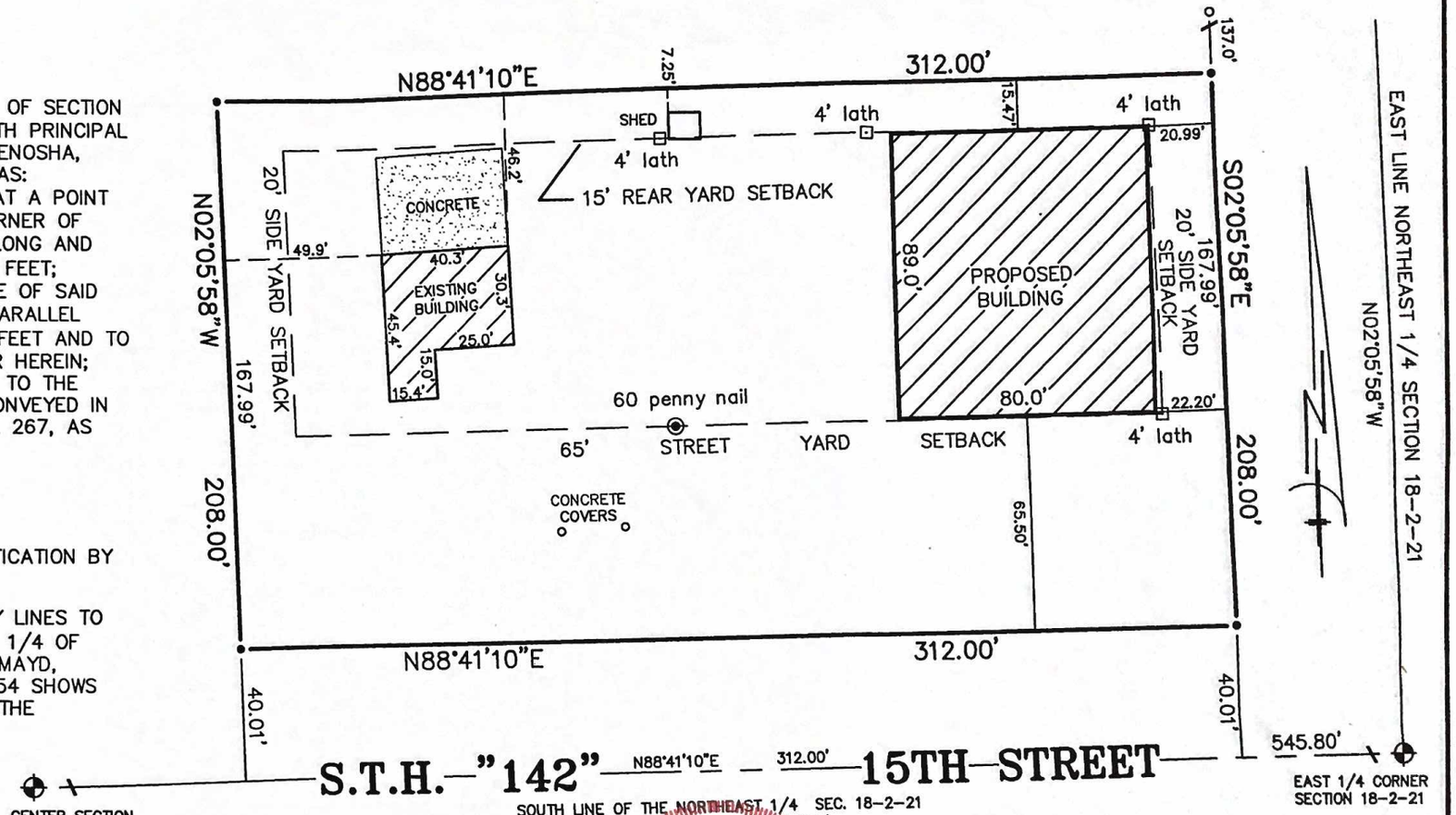
PLAT OF SURVEY
–OF–

PART OF THE SOUTHEAST 1/4 OF THE NORTHEAST 1/4 OF SECTION 18, TOWNSHIP 2 NORTH, RANGE 21 EAST OF THE FOURTH PRINCIPAL MERIDIAN, LYING IN THE TOWN OF PARIS, COUNTY OF KENOSHA, STATE OF WISCONSIN, MORE PARTICULARLY DESCRIBED AS: BEGINNING ON THE SOUTH LINE OF SAID 1/4 SECTION AT A POINT WHICH IS 545.80 FEET WEST FROM THE SOUTHEAST CORNER OF SAID 1/4, 1/4 SECTION AND RUNNING THENCE WEST ALONG AND UPON THE SOUTH LINE OF SAID 1/4, 1/4 SECTION 312 FEET; THENCE RUNNING NORTH PARALLEL WITH THE WEST LINE OF SAID 1/4, 1/4 SECTION 208 FEET; THENCE RUNNING EAST PARALLEL WITH THE SOUTH LINE OF SAID 1/4, 1/4 SECTION 312 FEET AND TO THE EAST LINE OF PROPERTY, OWNED BY THE GRANTOR HEREIN; THENCE RUNNING SOUTH ON SAID EAST LINE 208 FEET TO THE PLACE OF BEGINNING, EXCEPTING THEREFROM LANDS CONVEYED IN DEED RECORDED MAY 27, 1983, IN VOLUME 1134, PAGE 267, AS DOCUMENT NO. 703519.

SURVEY FOR: SCOTT BLUME
SURVEY ADDRESS: 20206 15TH STREET

NOTE: PROPOSED BUILDING TO BE STAKED UPON NOTIFICATION BY THE ABOVE NAMED CLIENT.

NOTE: DEED CALLS FOR THE NORTH–SOUTH BOUNDARY LINES TO RUN PARALLEL TO THE WEST LINE OF THE NORTHEAST 1/4 OF SECTION 18–2–21, PREVIOUS SURVEY BY HUGH SOUTHMAYD, KENOSHA COUNTY SURVEYOR DATED DECEMBER 21, 1954 SHOWS LOT LINES AS BEING PARALLEL TO THE EAST LINE OF THE NORTHEAST 1/4 OF SECTION 18–2–21.



"I hereby certify that I have surveyed the above described property and that the above map is a correct representation thereof and shows the size and location of the property, its exterior boundaries, the location of all visible structures and dimensions of all principal buildings thereon, boundary fences, apparent easements, roadways and encroachments, if any."

"This survey is made for the use of the present owners of the property, and those who purchase, mortgage, or guarantee the title thereto within one year from date hereof."

LEGEND

- FOUND KENOSHA COUNTY MONUMENT (CONCRETE/CAP)
- o FOUND IRON PIPE
- • SET IRON PIPE

GRAPHIC SCALE

50 25 0 50

SCALE: 1" = 50'

WISCONSIN
ROBERT J. WETZEL
S-1778
BURLINGTON, WI
LAND SURVEYOR

THIS IS NOT AN ORIGINAL PRINT UNLESS THIS SEAL IS RED.

B.W. SURVEYING, INC.
412 N. PINE STREET
BURLINGTON, WI 53105
(262)–767–0225

ROBERT J. WETZEL S–1778

REVISED: JUNE 8, 2020
JANUARY 14, 2020
DATE

10026
JOB NUMBER

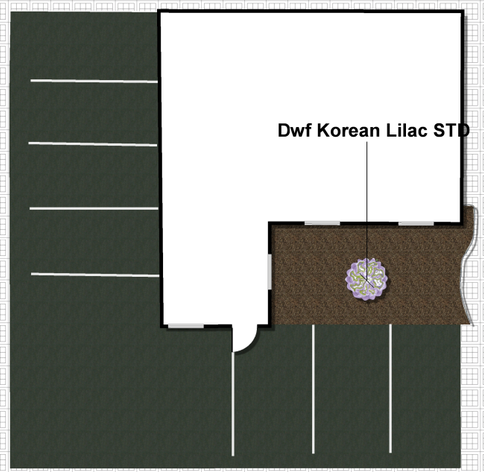
REVISED: JUNE 17, 2020

Blume Trucking
20206 15th Street
Union Grove, WI 53182

7-27-2016
Revised 7-27-20

Scale: 1"= 20'

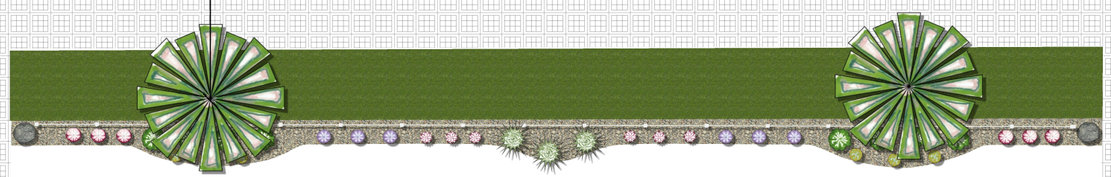
N



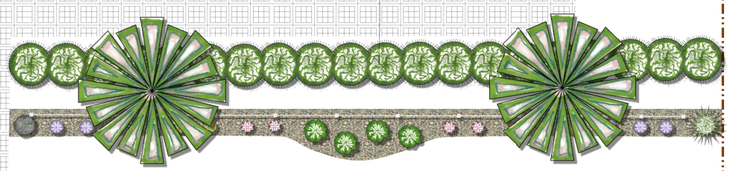
Dwf Korean Lilac STD

Proposed New Bldg
80' x 89'

4-Maple Trees



11-Techny Arborvitae



Hwy 142

PERIMETER WALL PACK

NRG®300 SERIES

Cat.#

Job

Type

HUBBELL Outdoor Lighting

Approvals

INTENDED USES

- Entry or perimeter security lighting applications for commercial buildings, shopping centers, schools, and apartment complexes

CONSTRUCTION

- Full polycarbonate front provides vandal resistance and efficient spread of forward and lateral light coverage
- Internally painted bronze finish for lasting appearance
- LED unit is an excellent upgrade from HID systems featuring a 17w LED system with a 1700+ lumen output, 4000K or 5000K CCT, 80 CRI - 60,500 hr rated life at L94
- Rugged cast aluminum back housing for rigid mounting; Bottom 1/2" conduit knockout for surface conduit wiring. Back hub allows access to recessed wiring boxes; Two point mounting; Additional center-pin torx screws provided for tamper-resistant applications

LED

- Single driver 50/60 Hz, 120-277V
- NRG-356L is 1858/1794 lumens, 4000K/5000K, 112/110 lPW, CRI 80, 1 driver at 100mA
- PC version 120V-277V

LISTINGS

- Listed to UL 1598 for use in wet locations
- Some LED models meet DesignLights Consortium (DLC) qualifications, consult DLC website for more details: http://www.designlights.org/QPL

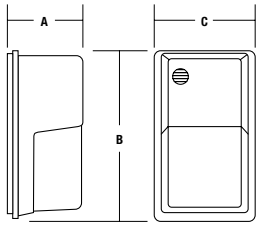
WARRANTY

For more information visit: http://www.hubbelllighting.com/resources/warranty/

PRODUCT IMAGE(S)

NRG-356LU-5K-BZ

DIMENSIONS



A	B	C
5.39" 137 mm	11.02" 280 mm	6.89" 175 mm

SHIPPING INFORMATION

Catalog Number	G.W(kg)/ CTN	Carton Dimensions			Carton Qty. per Master Pack
		Length Inch (cm)	Width Inch (cm)	Height Inch (cm)	
NRG356LU5KBZ	5.33 (2.42)	11.8 (30)	7.4 (19)	6.1 (15.5)	1
NRG356LU5KBZPC	5.40 (2.45)	11.8 (30)	7.4 (19)	6.1 (15.5)	1

CERTIFICATIONS/LISTINGS

tradeSELECT c UL us DLC QPL LISTED

ORDERING INFORMATION

Catalog Number[1]		Wattage	Voltage	Max Input Amps	Color	Weight lbs. (kg)
Without Photocontrol	With Photocontrol					
56 LED – 1858/1794 lumens – 4000K/5000K – 80 CRI						
NRG-356L-4K-U	**NRG-356L-4K-U-PC**	16.6w	120-277V	.15	Bronze	6 (2.7)
NRG-356L-5K-U	**NRG-356L-5K-U-PC**	16.3w	120-277V	.15	Bronze	6 (2.7)

ACCESSORIES - ORDER SEPARATELY

Catalog Number	Description
PBT-1	Button photocontrol, 120V
PBT-234	Button photocontrol, 208, 240, 277V

REPLACEMENT PART - ORDER SEPARATELY

Catalog Number	Description
SM352-COVER	Polycarbonate front cover, Bronze